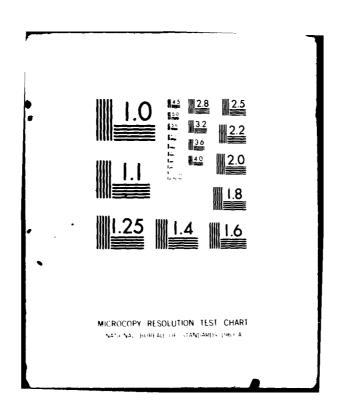
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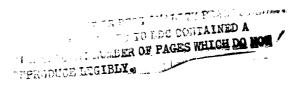
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 $^{2}$  LAKE CASSE DAM  $_{f}$ 

PUTNAM COUNTY, NEW YORK INVENTORY NO. N.Y. 1156

PHASE I INSPECTION REPORT

/ NATIONAL DAM SAFETY PROGRAM





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NEW YORK DISTRICT CORPS OF ENGINEERS

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SEPTEMBER 1981

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dam has some deficiencies which require further investigation and remedial action.

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The classification of "unsafe" applied to a dam because of a "seriously inadequate" spillway is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean that there appears to be a serious deficiency in spillway capacity with the steel box cover in place and that if a severe storm were to occur, overtopping and failure of the dam could take place, significantly increasing the hazard to loss of life downstream of the dam.

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# **LOWER HUDSON RIVER BASIN**

# LAKE CASSE DAM

PUTNAM COUNTY, NEW YORK INVENTORY NO. N.Y. 1156

# PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM



NEW YORK DISTRICT CORPS OF ENGINEERS
SEPTEMBER 1981

#### PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D. C., 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM LAKE CASSE DAM I.D. NO. N.Y. 1156 D.E.C. NO. 1797 LOWER HUDSON RIVER BASIN PUTNAM COUNTY, N.Y.

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## PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

NAME OF DAM

Lake Casse Dam

STATE LOCATED

New York

COUNTY LOCATED

Putnam

STREAM

TR Croton Falls Reservoir

BASIN

Lower Hudson River

DATE OF INSPECTION May 6, 1981

#### ASSESSMENT

The examination of documents and the visual inspection of Lake Casse Dam did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

Using the Corps of Engineers screening criteria for review of spillway adequacy, it has been determined that with the existing steel box cover in place the dam would be overtopped for all storms exceeding approximately 28 percent of the PMF. The overtopping of the dam could cause the erosion of both abutments and the downstream face of the dam resulting in dam failure, thus significantly increasing the hazard to the loss of life downstream. The spillway is therefore adjudged as "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate" spillway is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean that there appears to be a serious deficiency in spillway capacity with the steel box cover in place and that if a severe storm were

to occur, overtopping and failure of the dam could take place, significantly increasing the hazard to loss of life downstream of the dam.

It is therefore recommended that within 3 months of notification to the owner, detailed hydrologic/hydraulic investigations of the structure should be undertaken to more accurately determine the site specific characteristics of the watershed and their affect upon the overtopping potential of the The results of these investigations will determine the appropriate remedial measures which will be required to achieve a spillway capacity adequate to discharge the outflow from at least the one-half PMF. Within 12 months of the date of notification to the owner, modifications to the structure, deemed necessary as a result of studies, should have been completed. In the interim, the cover should be modified or replaced to allow full outlet capacity; a detailed emergency action plan must be developed and implemented during periods of unusually heavy precipitation. Also, around-the-clock surveillance of the structure must be provided during these periods.

In addition, the dam has a number of problem areas which, if left uncorrected, have the potential for the development of hazardous conditions and must be corrected within one year. These are:

let should be cleared of all brush, debris, and growth and the channel be cleaned out to a depth which will allow free flow from the discharge pipe. Following this clearing and channel cleaning, the seepage and flow co. tions in the area of the discharge pipe and toe should be examined and the condition of the outlet pipe up to the drop inlet structure should be investigated. Appropriate repairs, if required, should be determined and carried out.

- 2. The brush, saplings and debris should be removed from the downstream slope. All coniferous trees should be removed while larger hardwood trees should not be removed, but should be inventoried and their condition monitored. If a tree dies, the area around the tree should then be monitored for possible seepage. A program of periodic mowing and cutting should be provided.
- 3. All trees and brush on the upstream face should be removed and periodic mowing and cutting provided.
- 4. Provide a program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of the reservoir drain system. The results of the inspection and test operation should be documented for future reference. The aforementioned emergency action plan should be maintained and updated periodically during the life of the structure.

Eugene D'Brien, P.E. New York No. 29823

Approved by:

col. W.M. Smith, Jr.

New York District Engineer

Date:

14 avy 81



PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
LAKE CASSE DAM
I.D. NO. N.Y. 1156
D.E.C. NO. 1797
LOWER HUDSON RIVER BASIN
PUTNAM COUNTY, N.Y.

#### SECTION 1 - PROJECT INFORMATION

#### 1.1 GENERAL

#### a. Authority

The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers by Contract No. DACW 51-81-C-0008 dated 14 December 1980 in fulfillment of the requirements of the National Dam Inspection Act, Public Law 92-367, 8 August 1972.

# b. Purpose of Inspection

The inspection was conducted to evaluate the existing conditions of the dam, to identify deficiencies and hazardous conditions, to determine if these deficiencies constitute hazards to life or property, and to recommend remedial measures where required.

# 1.2 DESCRIPTION OF THE PROJECT

a. Description of the Dam and Appurtenant Structures
The Lake Casse Dam is composed of an approximately
690 foot long earth embankment with a crest width of 36 feet.
The maximum height of the dam is 14 feet. The upstream and
downstream slopes of the dam are 1V to 3H.

An outlet box located 33.5 feet upstream of the centerline of the dam and about 250 feet from the left abutment serves as a drop inlet spillway. The upstream face of the spillway has a 24-inch sluice gate which serves as a reservoir drain. The discharge from the outlet box enters a 48-inch reinforced concrete pipeline which is located under the dam. The pipeline exits through a headwall structure at the downstream toe of the dam. The top of the outlet box is 3 feet

below the crest of the dam and has a 5 foot by 5 foot opening. The opening is covered by a steel box which contains 3 side openings. Two of the openings are 2.5 feet wide by 7 inches high and the other is 4 feet wide by 7 inches high.

# b. Location

Lake Casse Dam is located on a tributary of the Croton Falls Reservoir about one mile northeast of the Village of Mahopac. Lake Road passes over the crest of the dam.

## c. Size Classification

The dam is 14 feet high and has a maximum storage capacity of 182 acre-feet and therefore is classified as a small dam.

# d. Hazard Classification

The dam is in the "high" hazard potential category because of its location directly upstream and in close proximity to a number of homes.

## e. Ownership

Lake Casse Dam is owned by the Lake Casse Property
Owners Association, P.O. Box 245, Mahopac, New York, 10541. The
officer of the Association contacted was Mr. Thomas Mulligan at Tel.
(914) 628-6244.

#### f. Purpose of Dam

The dam impounds water to provide a recreational lake for a housing subdivision.

# g. Design and Construction History

The dam was designed and constructed in 1953. The designer was Roy Burgess, Consulting Engineer, Main Street, Carmel, New York. The name of the contractor is unknown.

#### h. Normal Operating Procedures

There is no normal operating procedure nor records of past operating procedures. Reservoir level is changed on an asneeded basis by the Association members.

# 1.3 PERTINENT DATA

PER	FINENT DATA	
a.	Drainage Area, Square Miles	0.38
b.	Discharge at Damsite, cfs	
	5' Drop Inlet-Without Steel Cover	240
	5' Drop Inlet-With Steel Cover and Side Openings	41
	24-inch Sluice Gate	
	48-inch Reinforced Concrete Pipe	240
	Maximum Total Discharge Capacity (Reinforced Concrete Pipe)	240
c.	Elevation, Feet Above MSL,	
	USGS Datum	
	Top of Dam	610
	Maximum Design Pool	Unknown
	Spillway Crest	607
	Low Level Outlet	597.5
đ.	Reservoir	
	Length of Maximum Pool (Miles)	0.46
	Surface Area of Maximum Pool	
	(acres)	37.2
	Surface Area of Normal Pool	
	(acres)	30.3
e.	Storage, Acre-feet	
	Reservoir at Spillway Crest	80
	Reservoir at Maximum Pool	182
f.	Dam	
	Type	Earth Embankment
	Length (feet)	690
	Upstream Slope	1V:3H
	Downstream Slope	1V:3H
	Crest Elevation (MSL)	610
	Crest Width (feet)	36'
	Grout Curtain	Unknown
	Cutoff Trench	3.0' deep puddled clay
g.	Spillway	
	Туре	Uncontrolled Concrete
		Drop Inlet-Discharge
		through 48-inch RCP

Size

Drop Inlet (without cover)
Side Openings in Steel
Cover at Drop Inlet

Crest Elevation
Upstream Channel
Downstream Channel

5 ft Square

2 @ 2'-6" by 7" high 1 @ 4'-0" by 7" high

607.0 None

48-inch Diameter
Pipe Discharges
Through Headwall
Structure into Open
Channel

Auxiliary Spillway

None

h. Reservoir Drain and Pipeline

Reservoir Drain

24-inch Sluice Gate

Discharges into Bottom

of Drop Inlet at Elevation 597.5

Pipeline

Pipe in Common with

Spillway

#### SECTION 2 - ENGINEERING DATA

## 2.1 GEOLOGY

The records of the owner contain no data on site geology. However, there is data available in the literature on the general geology of the area. Lake Casse Dam is located in the Hudson Highlands section of the New England Uplands Physiographic Province. The province is characterized by a low, but rugged mountain range consisting primarily of igneous and metamorphic rock. The rock underlying the Lake Casse site is Precambrian biotite granitic gneiss.

#### 2.2 SUBSURFACE INVESTIGATIONS

There are no records of subsurface investigations carried out at the site. The "Application for the Construction" of the dam indicates the soil in the foundation and abutments to be "hardpan".

#### 2.3 DAM AND APPURTENANT STRUCTURES

There is a drawing showing the "General Plan" and several details of the dam; this is included in Appendix A. Additionally, of the original construction application is available and included in Appendix F.

#### 2.4 CONSTRUCTION RECORDS

No information has been located in relation to the construction of the project. The name(s) of the contractor(s) is (are) unknown.

#### 2.5 OPERATION RECORDS

There are no operation records kept for the dam. No systematic monitoring of the performance of the dam is in effect.

#### 2.6 EVALUATION OF DAM

There is sufficient data available to support a Phase I evaluation of the dam.

#### SECTION 3 - VISUAL INSPECTION

# 3.1 FINDINGS

#### a. General

The visual inspection of the Lake Casse Dam was made on 6 May 1981. The weather was overcast and the temperature was in the mid 60's. At the time of the inspection, the lake level was just above the spillway crest level.

#### b. Dam

The horizontal and vertical alignment of the crest of the dam show no signs of stress, deformation, or cracking. The upstream and downstream slopes of the earth embankment are heavily overgrown with trees and shrubs. The upstream slope of the dam suffers from local erosion at a level near the current water level.

The downstream slope of the dam appears to be somewhat irregular due to fill which has been placed to widen the crest subsequent to the construction of the dam. A complete view of the downstream slope and toe area was obscured by the dense brush and tree growth (See Photo No. 3).

There is no emergency action plan for the project.

#### c. Spillway

The drop inlet spillway, which was originally an open box culvert with 5 foot square opening, has been covered with a steel plate box with three small side openings. The steel plate box significantly reduces the discharge capacity of the inlet. The concrete sills of the spillway appear to be in good condition. Discharge was flowing through the side openings at the time of the inspection (See Photo No. 6).

The drop inlet spillway discharges through the 48-inch pipeline which is discussed in paragraph 3.1d.

#### d. Outlets and Pipelines

The 24-inch reservoir drain, which is located in the drop inlet tower at a depth below crest level of 12.75 feet, was unobservable. The sluice gate is however, reportedly operable and is used whenever required to lower the reservoir.

The sluice gate and drop inlet spillway discharge through a 48-inch reinforced concrete pipeline. The downstream exit of the pipeline is clogged for about 3/4 of its depth by silting and debris. This severely restricts the flow capacity of the pipeline. At the time of the inspection, the water entering the spillway could not be seen discharging at the silted-up pipe exit, but the discharge could have been observed in the channel about 50 feet downstream of the pipeline exit (See Photos No. 8 and 9).

#### e. Abutments

The abutment/dam contacts and abutments are in good condition. There does not appear to be any low point or erosion in these areas.

#### f. Reservoir Area

The reservoir is located in a hilly lightly developed residential area. No slides, rock falls, or sloughing were observed around the reservoir. There is no visible sediment accumulation in the reservoir.

## 3.2 EVALUATION OF OBSERVATIONS

Although deficiencies were observed, there is no indication that the dam is in imminent danger. A number of the deficiencies observed in the previous paragraphs are minor and can be corrected by increased maintenance. Other conditions described above, however, represent conditions which may have potential for further deterioration and for this reason these conditions need to be investigated or corrected.

The most significant feature requiring correction and further investigation is the condition of the discharge area of the 48-inch low level outlet pipe. Previous reports in 1969 (see Appendix F ) have stated that although water was entering the drop inlet structure, none was exiting from the 48-inch outlet pipe downstream of the dam. Therefore, it was concluded that the pipe was broken within the dam allowing the water to exit through some crevice in the foundation and in due time could lead to failure of the dam. It also reported that the downstream slope and toe area of the dam were unobservable due to excessive brush growth.

As reported in Section 3.1c and 3.1d, these conditions still exist. However, at the time of the inspection, water was flowing in the discharge channel about 50 feet downstream of the dam at about the same rate as water was entering the drop inlet. In view of the existence of the condition for 10 years and the fact that there is no evidence of cracking, sloughing or slides on the dam the previously reached conclusions regarding the broken pipe may not be correct. However, it is still not possible to clearly identify the discharge conditions which exist at the pipeline.

It is recommended, therefore, that the downstream discharge area be cleared of all brush, debris and growth and the channel be cleaned out to a depth which will allow free flow from the discharge pipe. Following this clearing and channel cleaning of the debris, the seepage and flow conditions in the area of the discharge pipe and the toe should be examined and the condition of the outlet pipe through its entire length, should be investigated. Solutions appropriate to the problems identified, if any, should be determined and carried out.

Other significant deficiencies observed, which require immediate corrective action to insure the safety of the dam are listed below. The following is a summary of the problem areas encountered with the appropriate recommended action.

- l. The steel box cover which has been placed over the drop inlet has significantly reduced the spillway capacity. This cover should be removed and be replaced by an appropriate trash rack structure which will provide maximum spillway capacity.
- 2. All brush, saplings and debris should be removed from the upstream and downstream slopes. All coniferous trees should be removed while larger hardwood trees should not be removed but should be inventoried and their condition monitored. If a tree dies, the area around the tree should then be monitored for possible seepage. A program of periodic cutting and mowing should be provided.

- 3. The local erosion on the upstream face should be repaired by regrading the area. Slope protection should then be provided to prevent a reoccurrence of the erosion.
- 4. A program of periodic inspection and maintenance of the dam and appurtenances including yearly test operation and lubrication of the gates should be provided. The results of the inspection and test operation should be documented for future reference. An emergency action plan described in Section 7.1d should be established, maintained and periodically updated during the life of the structure.

#### SECTION 4 - OPERATION AND MAINTENANCE PROCEDURES

# 4.1 PROCEDURES

No written operation and maintenance procedures exist for the project. There is no scheduled operation of the project.

#### 4.2 MAINTENANCE OF THE DAM

There is no regular maintenance schedule for the dam and no regular maintenance is performed on the dam or appurtenances.

# 4.3 WARNING SYSTEM IN EFFECT

No warning system is in effect or in preparation.

# 4.4 EVALUATION

The overall maintenance of Lake Casse Dam is considered inadequate in the following areas:

- 1. The area of the low level outlet discharge is clogged with brush, silt and debris.
- 2. The upstream and downstream slopes are not kept clear of brush, trees and debris. The upstream slope is subject to erosion.
- 3. No formal operation and maintenance manual exists for the project.

#### SECTION 5 - HYDROLOGIC/HYDRAULIC

# 5.1 DRAINAGE AREA CHARACTERISTICS

The Lake Casse Dam is located at the southern end of Lake Casse in Carmel Township, Putnam County, New York (Hydrologic Unit Code 02030101). The area of the drainage basin is 0.38 square miles, with a maximum (east-west) width of 0.6 miles and a maximum (north-south) length of 0.8 miles. The area is moderately developed, with low rolling hills, and wide valleys. Land cover ranges from lawns to woodlands, and rises from a lake elevation of 607 feet (MSL) to just over 750 feet in the northwestern corner of the basin.

# 5.2 ANALYSIS CRITERIA

The analysis of the adequacy of the spillway was performed by developing a design flood, suing the unit hydrograph method, the Probable Maximum Precipitation (PMP) and the HEC-1DB computer program. The all season 200 square mile 24 hour PMP for the Lake Casse area of 22 inches was obtained from Hydrometeorological Report No. 33. The unit hydrographs were computed using Snyder's method and coefficients of 2 and 0.5 for C<sub>t</sub> and C<sub>p</sub>, respectively. Rainfall loss parameters of 1.0 inch initial loss and 0.1 inch per hour constant loss were assumed.

In accordance with the Recommended Guidelines for Safety Inspection of Dams (Ref. 1), the adequacy of the spillway was analyzed using the Probable Maximum Flood (PMF). Two multi-plan analysis were performed using 0.25, 0.50, 0.75 and full PMF for the spillway with, then without the steel cover.

#### 5.3 SPILLWAY CAPACITY

An outlet box located 33.5 feet upstream of the centerline of the dam 250 feet from the left abutment serves as a drop inlet type spillway. The sill level for the spillway is 3 feet below the crest of the dam at El 607. Total sill length as designed and built is 20 feet. The spillway discharges through a 48-inch concrete pipe under the dam. There is currently a steel box cover with 3 small side openings over the top of the drop inlet significantly reducing the spillway capacity.

The computed maximum discharges of the spillway with and and without steel cover are 41 cfs and 240 cfs respectively when lake surface is at El 610 (top of dam).

# 5.4 RESERVOIR CAPACITY

The normal capacity of the reservoir is listed as 80 acrefeet. The computed surcharge storage between spillway crest elevation 607 feet and top of dam El 610 is 102 acre-feet, which is equivalent to about 5 inches of runoff over the entire basin.

#### 5.5 FLOODS OF RECORD

There are no records of floods or maximum reservoir elevations at the dam.

## 5.6 OVERTOPPING POTENTIAL

The potential of the dam being overtopped was investigated on the basis of the spillway discharge capacity and the available surcharge storage to meet the selected design flood inflows.

The analysis was performed assuming that the water surface in the reservoir was at the spillway crest elevation at the start of the flood event. The results of the multi-plan analysis are shown below.

# WITH COVER

Ratio of PMF	Inflow Peak (cfs)	Outflow Peak (cfs)	Overtopping (ft)
1.00	911	898	0.54
0.75	683	667	0.44
0.50	455	367	0.28
0.25	228	35	0.00

Spillway passes 27.5% PMF without overtopping

WITHOUT COVER			
1.00	911	888	0.45
0.75	683	545	0.27
0.50	455	233	0.00
0.25	228	202	0.00

Spillway passes 53.6% PMF without overtopping

# 5.7 EVALUATION

With the steel box cover in place, the Lake Casse Dam spillway is unable to pass either the PMF or one-half (1/2) PMF without the dam being overtopped. The overtopping of the dam could cause the failure of the dam, thus significantly increasing the hazard for the loss of life downstream. The spillway is therefore assessed as being "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

#### SECTION 6 - STRUCTURAL STABILITY

# 6.1 EVALUATION OF STRUCTURAL STABILITY

#### a. Visual Observations

Visual observations did not indicate any structural problems with the embankment or appurtenant structures with the reservoir at its present level. There are no adverse conditions which would affect the stability of the dam at its present level. As detailed in Section 3, however, an important question exists with regard to the path of the discharge from the 48-inch low level outlet and the hydraulic capacity of the drop inlet spillway structure with its cover in place. During flood conditions these situations increase the potential for overtopping and erosion of the downstream face of the dam.

# b. Design and Construction Data

A design drawing has been located for the project. A review of this drawing does not reveal any potential structural stability problems.

## c. Operating Records

There are no operating records presently kept or available. There are no records or reports of any operational problems which would effect the stability of the dam.

#### d. Post-Construction Changes

There are no reported post-construction changes to the dam or appurtenant structures. There was however the addition of a metal box structure over the drop inlet spillway as described in Section 3. The addition of this metal box structure significantly reduces the spillway capacity and thus during flood conditions increases the potential for overtopping and failure of the dam.

#### e. Seismic Stability

In accordance with recommended Phase I guidelines, the dam is located in Seismic Risk Zone No. 1. However, based on past local seismic experience, the New York State Geological Survey recommended that the damsite is to be considered in Zone 2.

#### SECTION 7 - ASSESSMENT/RECOMMENDATIONS

# 7.1 ASSESSMENT

# a. Safety

Examination of the available documents and visual inspections of the Lake Casse Dam and appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

Using the Corps of Engineers screening criteria for review of spillway adequacy, it has been determined that with the steel box cover in place that the dam would be overtopped for all storms exceeding approximately 28 percent of the PMF. The overtopping of the dam could cause the erosion of the abutments and the downstream face of the dam resulting in dam failure, thus significantly increasing the hazard for loss of life downstream. The spillway is therefore adjudged as "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate" spillway is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however, that based on an initial screening and preliminary computations, that with the steel box over in place, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

#### b. Adequacy of Information

The report and its conclusions are based on a visual inspection, interview data, contract drawings, and office hydrologic and hydraulic studies. This information and data are adequate for a Phase I inspection.

#### c. Need for Additional Investigations

Since the spillway is considered to be "seriously inadequate" with the metal box cover in place, additional hydrologic/

hydraulic investigations are required to more accurately determine the site specific characteristics of the watershed. After the in-depth hydrologic/hydraulic investigations have been completed, remedial measures must be initiated to provide spillway capacity sufficient to discharge the outflow from the one-half (1/2) PMF event.

In addition, as described in Section 3.2, an investigation should be carried out to study the low level outlet discharge conditions and to carry out the appropriate remedial measures.

## d. Urgency

The additional hydrologic/hydraulic investigations which are required must be initiated within 3 months from the date of notification. Within 12 months of notification, remedial measures as a result of these investigations must be initiated, with completion of these measures during the following year. In the interim, develop an emergency action plan for the notification of downstream residents and proper governmental authorities in the event of overtopping, and provide around-the-clock surveillance of the dam during periods of extreme runoff. The other problem areas listed below must be corrected within one year from notification.

#### 7.2 RECOMMENDED MEASURES

Recommended measures are as follows:

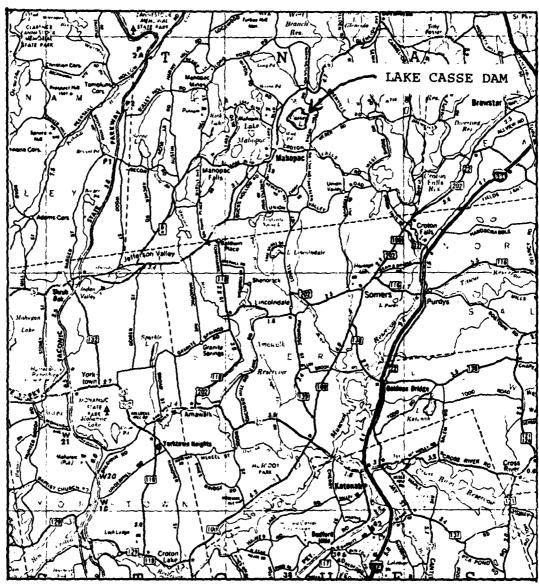
- 1. The steel box cover which has been placed over the drop inlet should be removed and replaced by an appropriate trash rack structure which will provide maximum spillway capacity.
- 2. All brush, saplings and debris should be removed from the downstream slope. All coniferous trees should be removed, while larger hardwood trees should not be removed but should be inventoried and their condition monitored. If a tree dies, the area around the tree should then be monitored for possible seepage. A program of periodic mowing and cutting should be provided.

- 3. The local erosion on the upstream face should be repaired by regrading the area. Slope protection should then be provided to prevent a reoccurrence of the erosion.
- 4. A program of periodic inspection and maintenance of the dam and appurtenances including yearly test operation and lubrication of the reservoir outlet system should be provided. The results of the inspection and test operation should be documented for future reference. The emergency action plan described in Section 7.1d should be maintained and updated periodically during the life of the structure.

# DRAWINGS

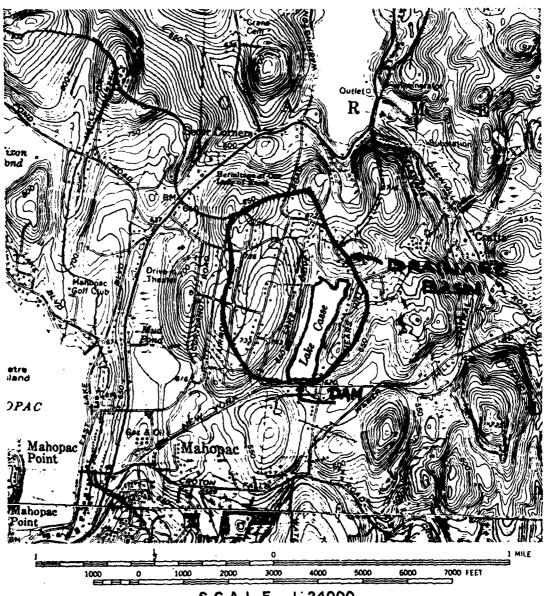
- a. Vicinity Map
- b. Topographic Map
- c. Plan Sections and Details

APPENDIX A



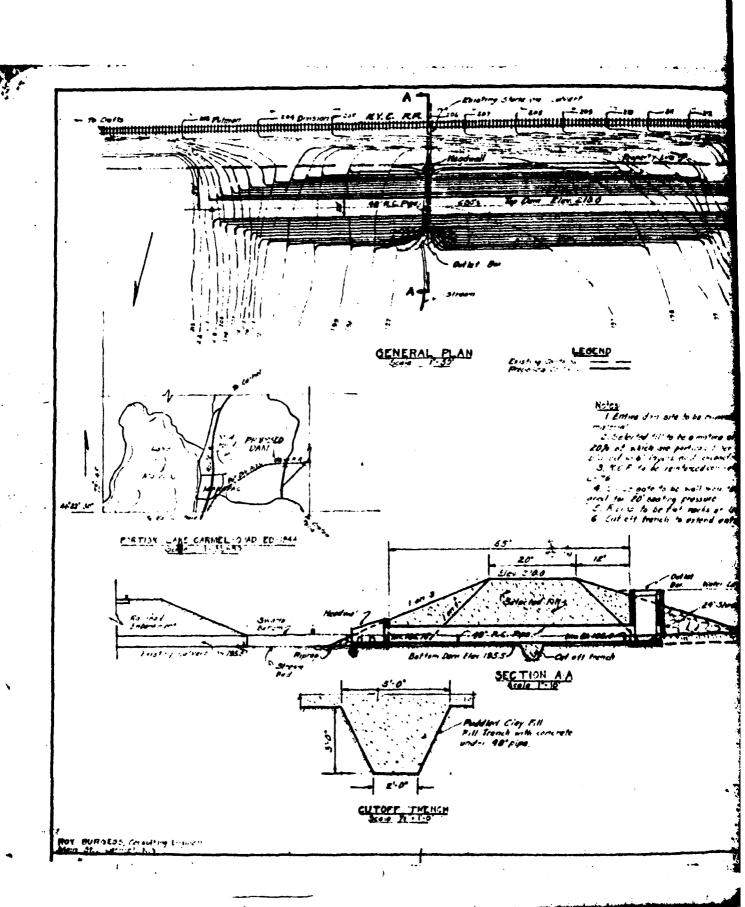
Scale: 1"=2.2 Miles

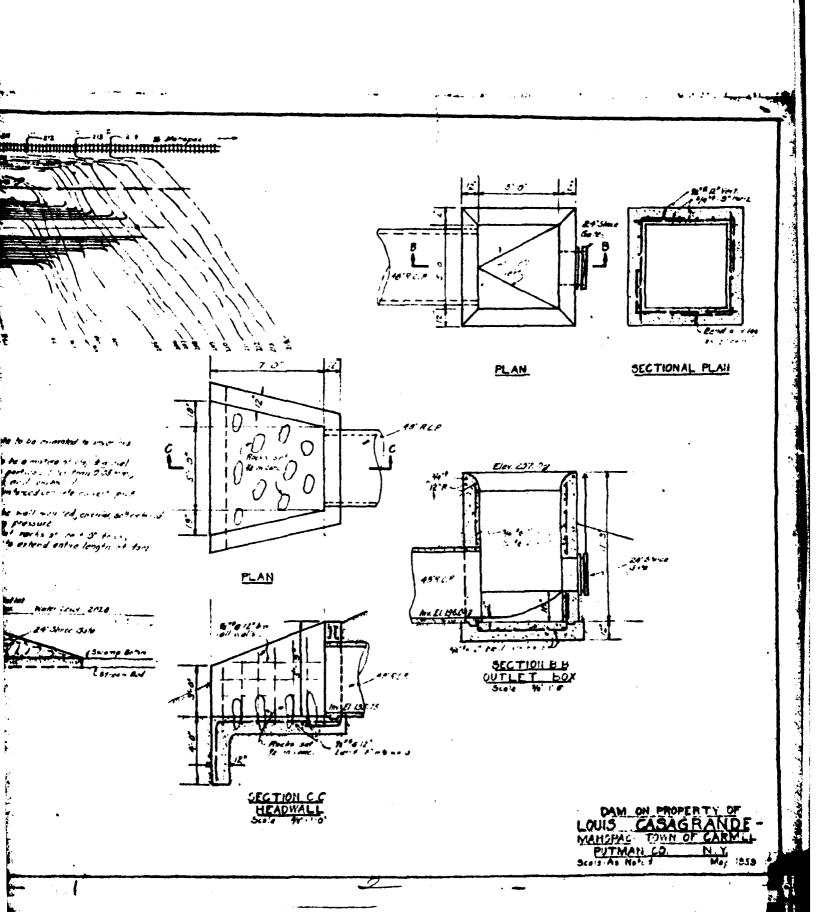
LAKE CASSE DAM
VICINITY MAP



SCALE 1:24000

TOPOGRAPHIC MAP LAKE CASSE DAM





**PHOTOGRAPHS** 



2. VIEW ALONG CREST OF DAM FROM RIGHT ABUTMENT



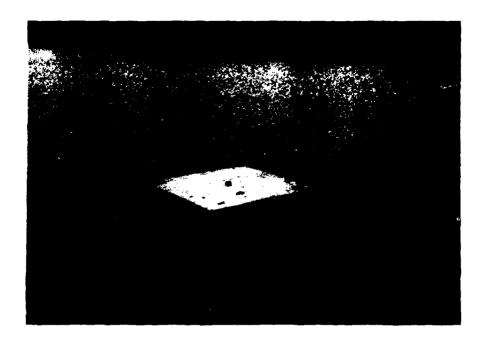
3. VIEW OF DOWNSTEPAM PACE OF DAM



4. VIEW OF UPSTREAM FACE OF DAM



5. Choseup of uprations there of plot



6. DROP INLET SPILLWAY/OUTLET STRUCTURE



7. COTT OUT OUT OF PIPE



8. CLOSEUP OF OUTLET PIPE



9. DISCHARGE
CHARKET
50 PERT DOWNS
STREAM OF OUTLUT PIPE

VISUAL INSPECTION CHECKLIST

### VISUAL INSPECTION CHECKLIST

Вa	si	c	Da	ta

a.	General
	Name of Dam Long Pond Dam No. 3
	Fed. I.D. # NY 00 115 DEC Dam No. /
	River Basin Manys River
-	Location: Town North Castle County Westchestee
	Stream Name <u>Unwanted Depinage</u>
	Tributary of Miamus River
	Latitude (N) 41-08.5 Longitude (W) 073-40.4
	Type of Dam EARTH AND Rockfill
	Hazard Category HIGH (I)
	Date(s) of Inspection 9 June 1981
	Weather Conditions Ovarious & intermitting showers
	Reservoir Level at Time of Inspection 470,5= (ESTIMATED BASEDOW MS65 MAP)
ь.	Inspection Personnel HARVEY Fellowers - Pariscipal Georgian Eng.
	John F. Wallace-Georgehical Engineer
c.	Persons Contacted (Including Address & Phone No.) Mr. K. Karl Mueller
•	LONG POND Ct., WINDMILL FARMS, ARMONK, N.Y. 10504
-	(914)-273-8074
•	
d.	History:
•	Date Constructed Unknown (Creen 1935) Date(s) Reconstructed
	Designer Elwyne E. SEELYK (Co., Consulting Engineer N.V., N.Y.
	Constructed By UNKNOWN
	Owner MR. K. KARI Mueller, Longfond Ct. ARMONE, N.Y.

a.		racteristics			
	(1) Embankment Material EARTH AND Rock Fill				
	(2)	Cutoff Type None known to Exist			
	(3)	Impervious Core None Known to EXIST			
	(3)	Impervious core //voio & zoosovo			
	(4)	Internal Drainage System Nowe KNOWN to EXIST			
	(5)	Miscellaneous			
ı.	0				
Д.	b. Crest  (1) Vertical Alignment 9000				
	(2)	Norizontal Alignment 9000			
(3) Sur		Surface Cracks None			
	(4)	Miscellaneous			
c.	Upst	ream Slope			
•	(1)	Slope (Estimate) (V:II) / (Y:5H to IV:6H			
	(2)	Undesirable Growth or Debris, Animal Burrows Numerous bushes  Occanoual small (saple than 8" Jam) decipyons trees			
	(3)	Sloughing, Subsidence or Depressions None observed			
• . •	•				

Embankment

(5)	Surface Cracks or Movement at Toe NONG observed
Dowr	stream Slope
(1) (2)	Slope (Estimate - V:II) Broken Slope Wils H to /V:2.0H w/20 bench Near North Admin Spilling Charvel structured Authority Charvel structured Burdens Number of Microbe
	Ani (8th 18" diposter) decionous tras - husbens hents AT acor
(3)	Sloughing, Subsidence or Depressions
(lt)	Surface Cracks or Movement at Too
(5)	Scepage sarpage at MIDS love 1 SOUTH Abut MINT - SEE SU flow ESTIMATED to be Sto 7 grin. NO SOIL EROSION
(6)	External Drainage System (Ditches, Trenches; Blanket) spillway  Chapped lowed on Downstrain slope wair exert: Asurinsen
<b>(7)</b>	Condition Around Outlet Structure <u>some wares custing of sould</u>
	Seepage Beyond Tue None observed

•	(1)	Erosion at Contact //2/12
	(5)	Scepage Along Contact AT INDONUT of NEST ABUTINIST -  granning 25T be be 5 to 79pm
•		
Dra	inage	System
a.	Desc	ription of System NONE
	•	
ъ.	Cond	ition of System
•	•	
c.	Disc	harge from Drainage System 10/A
•		
<u>Ins</u> Pi	trume ezome	ntation (Momumentation/Surveys, Observation Wells, Weirs, ters, Etc.) <u>Nowe Observe</u>
<del></del>		
<del></del>		•
		•
•		

	cervair
a.	Slopes Appear to be growth smake in sum of
	slousing or insmoility in UKCINITY of DAM OR SOUTHS
٥.	Sedimentation None
2.	Unusual Conditions Which Affect Dam Location of North Lake Dam
	immenialy Mosmonn
ro	a Downstream of Dam
а.	Downstream Hazard (No. of Homes, Highways, etc.) Souchal Residents
	komp -
b.	Seepage, Unusual Growth None
٠	
e.	Evidence of Movement Beyond Toe of Dam None
d.	Condition of Downstream Channel NATURA SWAI WITH few frees
	OTHERWISE GOOD
Sp:	llway(s) (Including Discharge Conveyance Channel)
a.	General Rectangualar Reinforces Box Culvert Locates
•	AT NORTH END of DAM - 4 high by 6 wice SECTION 54 in
•	length inclining Approach And discharge 5/ABS - Walls AR
	1ft flick WITH
b.	Condition of Service Spillway sone spalling of Texining walls and
	Appende Epges - bottom star ON ChUINSTISAN TIP IS
	condend and partielly separated apparenting 3-5 (est
÷	from one some under worming of she support
	has resulted

c.	Condition of Auxiliary Spillway ROMDWAY CONFINENCE TO TOPE A DE
	sour ero. of Lake may from a crest chargeon partition feet
	lower the fic outral more clest in trater could function
	115 A Seconery sallway under perthon (PMF) consistons
•	PRINTED IS GOOD INCH SUSPENTS IN ASOLALEDANS TONO
d.	Condition of Discharge Conveyance Channel Channel Section ON
	DOWN STREAM DANT FACE hepsily RELETED WITH SHEAR MASSIVE
	Coulders - NO SIGNIFICANT & ROSION - SECTION begans toe
	in NATHER Channel - De laquely clear except for occasional
	decipious trees - 9000 CONDITION
Res	ervoir Drain/Outlet
	Type: Pipe //nknown Conduit Other
	Material: Concrete Un Known Metal Other
<b>.</b> .	Size: // NKNOWN Length UNKNOWN
•	
	Invert Elevations: Entrance UNK NOWN Exit UNKNOWN
•	Physical Condition (Describe): Unobservable X
	Material: Mukrown
	Joints: Alignment
	Structural Integrity: unknown
	•
	Hydraulic Capability: UN KNOWN
	nyur mazzo dapanaza di Constanti di Constant
•	Means of Control: Gate ? Valve ? Uncontrolled
	Operation: Operable Inoperable Other WNKNOWN
	Present Condition (Describe): Outlet clownstrain of dem 15 presum
	Present Condition (Describe). Option Come 21/2414 of Cipan 13 Massing
•	burrie un per several procebulders - gare house in simuted in Rosace

a.	Concrete Surfaces See ITEM 7 Southers
b.	Structural Cracking Sel Mem 76
c.	Movement - Horizontal & Vertical Alignment (Settlement)
	See Then 76
d.	Junctions with Abutments or Embankments spillury junctions in its
	de de des de
	Athe Comme stee- une come in ance more
	the Commester- Mereconnec for oneco Moore
	Insin to come.
- e.	Drains - Foundation, Joint, Face No No. 06301001
•	
£	Water Passages, Conduits, Sluices <u>Sep III.17</u>
I.	
Ι.	
Ι.	
Ι.	
•	
•	
•	Seepage or Leakage Leakage be low descursion sollie
•	Scepage or Leakage Jephys be for downstrum softe Channel s/AB throw out one severnor be
•	Scepage or Leakage <u>leakage below descursancian souther</u> Channel stab throw out one sesermon bes  446 feet upsmin of denominancial

	Joints - Construction, etc. Nove Obsarray
•	
•	
•	
,	Foundation
•	·
,	
	·
•	
4	Abutments
	•
•	
	Control Gates None
٠	,
Ì	Approach & Outlet Channels
_	
•	
-	· · · · · · · · · · · · · · · · · · ·
•	Energy Dissipators (Plunge Pool, etc.)
•	
	Intake Structures NOT Obscensele
-	
•	
•	
	Stability <u>gano</u>
•	
-	discellancous
-	Miscellancous

a. Descrip	otion and Condition
	See item 7 Spillway NO OTHER ApplINTENIANT STEE
<del></del>	NO OTHER A POURTENANT STRA
•	Were Absertion
	The Control
·	
•	
•	
<del></del>	

HYDROLOGIC DATA AND COMPUTATIONS

# LONG POND DAM #3.

# CHECK LIST FOR DAMS HYDROLOGIC AND HYDRAULIC - ENGINEERING DATA

#### AREA-CAPACITY DATA:

		Elevation * (it.)	Surface Area (acres)	Storage Capacity (acre-ft.)
1)	Top of Dam	476	11.28	173.6
2)	Design High Water (Max. Design Pool)	Mukapun		
3)	Auxiliary Spillway . Crest	Low highway es	n benkines	
4)	Pool Level with Flashboards			
5)	Service Spillway Crest	410	.826	115

Volume

#### DISCHARGES

•		•	(cfs)
1)	Average Daily	`	UNKNOWN
2)	Spillway @ Maximum High Water	•	222
3)	Spillway @ Design High Water		UNKNOWN
4)	Spillway @ Auxiliary Spillway Cres	st Elevation	UNKNOWN
5)	Low Level Cutlet	•	UNIKNOWN
6)	Total (of all facilities) @ Maximu	um High Water	2.7.2.4
7)	Maximum Known Flood		UPKNOWN
3)	At Time of Inspection		NAKHOWN

<sup>\*</sup> All Elevations Are bosed on Pour Level Dates Monsy Lake, NY.
Relative to AN elevation interpolated From the Wises Monsy Lake, NY.
QUAP. Sheet.

Enteritand CREST:	:		ELEVATION: _	476
Туре:	Enera Ho Vacto Sill			
	34'	Length:	365	/
	NONE			
Location	-			
SPILLWAY:		·		
SER	wicę		VAXIL	IARY
4	10" INVERT Elev	ation	······································	
-40 HX 60	o'w box eulvert Ty	'pe		
Approx	54.0 inlength wid	th		
	Type of C		•	:
	Uncontr	olled	•	-
	Contro	led:	•	
	(Flashboards	pe ; gate)		
	Numbe	<b>\</b>	•	
	Size/Le	ength \		
•	Invert Mat	erial		
	Anticipated of operating			
	Chute Le	ength	<u> </u>	
•	Height Between & Approach Ch (Wei⊅	annel Inver		

HYDROMETEROLOGICAL GAGES: NONE USED
Type:
Location:
Records:
Date
Max. Reading
Warning System: None
Method of Controlled Releases (mechanisms):
GATE VALUE ON RESERVO in DIFFIN

NINAGE AREA:			.0.52	
ATNAGE BASIN RUNOFF	CHADACTEDISTICS			
	•		<u></u>	
Land Use - Type:		- SMALL	+# BIVE	<del></del>
Terrain - Relief:	· 11 1004			<del></del>
Surface - Soil:	GLACIAL	· TILL		
Runoff Potential (	(existing or plan (surface or subsu			existing
· ·		0-555-60		•
	700112	0 33210		
•				
e ·				
Potential Sediment	No	nc.	•	
Potential Backwate	r problem areas		aximum storage	capacity
Potential Backwate			aximum storage	capacity
Potential Backwate	r problem areas		aximum storage	capacity
Potential Backwate	r problem areas		aximum storage	capacity
Potential Backwate including su	r problem areas ircharge storage:	for levels at n		
Potential Backwate	r problem areas ircharge storage:	for levels at n		
Potential Backwate including su	r problem areas ircharge storage:	for levels at n		
Potential Backwate including su  Dikes - Floodwalls Reservoir pe	r problem areas ircharge storage:	for levels at n		
Potential Backwate including su  Dikes - Floodwalls Reservoir per Location:	r problem areas ircharge storage:	for levels at n		
Potential Backwate including su  Dikes - Floodwalls Reservoir pe Location: Elevation:	r problem areas ircharge storage: (overflow & non- rimeter:	for levels at n	ow reaches alo	ing the

TAMS	
Jub No. 1579-10  Project LONG POND DAM NO 3  Subject Hydrologic / Hydrologic Computations  Hydrologic Unit Code No 01100006	Sheet 1 of 85  Date JUNE 18, 81  By
Assume:	
1. BASAN UPSTREAM OF WINDMILL LAKE I CONTROLLED & WILL NOT CONTRIBUTE to	S COMPLETELY LONG POND INFLO
2. For Analysis Basin is divided into	3 Sub-acros
in North Lake sub-area	
(a) Sub-acco - 1 downstroom of North take	٠
3) Subonce 2 West of Long Pour cate with TOTAL AREA 20.46 SQMILES.	g 35 Williamite Fries
JUB. AREA ! (59.2 ac + 0.09 mi)	
L = 1900' = 0.36 miles	
Les 400' - 0.076 miles	
Use C1 = 2 & 640 Cp = 400 C1 - 0.625.	
t = 2 (.36 x 0.076) = 0 68 hrs	
the 06/50 = 0.12 his	

Sus. AREA 2 (67.5 acres : 0. 11 mi2)

Use 
$$C_7 \cdot 2 \neq 64(C_7 \cdot 400)$$
  
 $t_7 \cdot 2 \left(0.3 \times 0.1\right)^{5/2} = 0.70 \text{ km}$   
 $t_7 \cdot 3.70/0.5 = 0.13 \text{ km}$ 

		Ch'k. by
Subject	HYDROLOGIC/HYDRAULIC COMPUTITIONS	By
Project	LONG POND DAM #3	Date
Job No.	1579-10	Sheet 2 of

SUB AREA 2 CONT 96 IMPORTIONS 4.13 0.06

tp= tp + 0.25(tp-tp)

FOR tp= 0.33 hrs

tpR= 0.72 + 0.25(0.33-0.13) = 0.77 hours

NORTH LAKE Sub-basin (from North Lake DAM \$1.3)

(PHASE 1 REFORT)

tpr = 0.98 hrs

Job No. 1579 - 10	Sheet of
Project Long Pous DAM No 3	Date June 18, 81
Subject HYDRAULIC	Computational DLC.
	Ch'k, by \( \sqrt{M.D.}

SPILLWAY RATING INVERT EL 410. USE MANNING FORMULA for depte de 1 2,35 Q. 149 ART S/2 S= 0.0025 S = 0.0025 d .05 24.6 99.3 6. 8 .15 1.28 67.3 2 12 10 1.2 13 1.6 3.5 15 142.6 21 d. 4. b. 6 Q1 : (See amories chart, Open - CHAMMER HYDRAUMES - CHEW) H/d H 24 0.3 1. 2. 471.2 0.5 2.0 8,8 53 412 474 Too allow collect. 1.0 4-25 150 5 1.25 30 180 475 5.5 () 33.5 57 1.375 201 475.5 TOP OF DAM EL 476 1.5 222 2.0 8 270 478 2.5 518 10 480

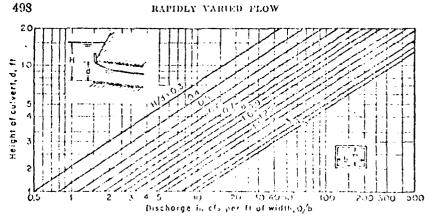


Fig. 17-29. Chart for estimating headwater on box culverts with square-edged entrances, flowing partly full. (Based on data of U.S. Eur. 11 of Public Rands (2011)

Job No. 1579 - 10

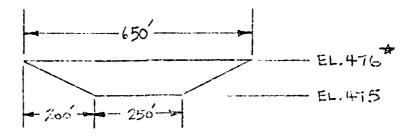
Project Louis Print Dans No. 3

Sheet 4 of 85 Date 6/25/21

Subject LOW Spot in REVERYOIR WHICH ACTS HIS

By -110 - 510

SECONDARY SPILLWAY - DESHARGE VS ELEVATION Ch'k. by



ASSUME: N=0.025 (LAWN & ROADWING SURFACES)

ELEV.	A ( <u>(</u> ; <del>-</del> ( <u>+</u> <sup>2</sup> ))	<u>P</u>	R (E)	Rich	Q= 1.49 AR3 S/= 1.9072 AR3
475					0
475.5	175	450	0.39	0.53	ハマフ
476	450	650	0.69	0.78	669
478	1750.	654-	2.68	1.93	6433

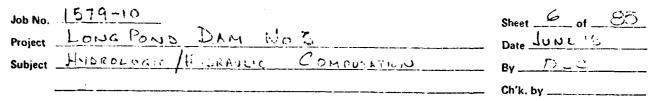
\*ABOVE EL. 476, THE SPILLWAY WIDTH WILL REMAIN AT 650 FT.

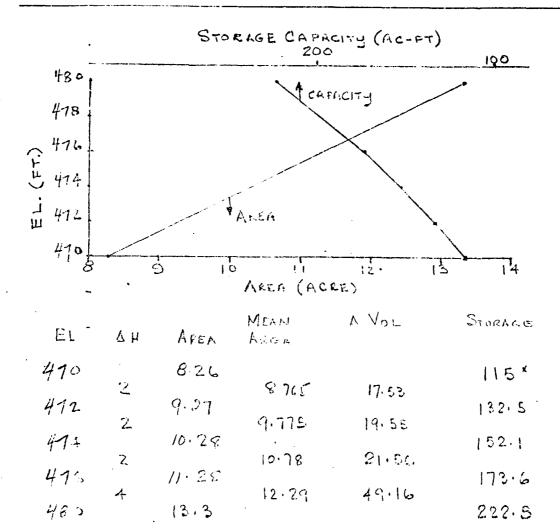
NORTH LAKE (ID # 113) - STORAGE: MEANIAPEN A VOL HA WOITHVELL STORAGE ARLA AC. AG-FT: 11-11 210 572 21.6 22.25 44.5 254.5 574 22.9 23.25 23.25 515 277.75 23.6 24.3 48.6 2. 326.15 25.0 577 26.55 17:15 3 404.5 580 27.1

Job No. 1572-13	Sheet 5 of
Project Complain Project	Date
Subject Andrew Programme Subject	By
	Ch'k. by

Long bond -	- SPILLWAY	(SE Corner of lake)	
Eliscikins	CRIMARY SPILLIA	FLOW OVER LOW SPOT	TOTAL Q
410		ر. ق	0
471.2- 472.	24 53	0	24 53
474.	150	0	150
475	180	5	185
475.5	201	177	378
476	222	669	891
사기 8 ★ Y4I	2.7 0	6 433	6703 4 45

	PILLWAY DISCHARCE	E DAM)	BREADTH	≈ 6 o'
Q = CLH <sup>3</sup> /			7.0	
	(CFS)	L H C.	Q,	Q,
573	3 0 0 3 1 2.68 B	0 0	12.9	20.9
573.75	3 175 2 65 18.4	14 1.25 2.66	52.0	70.4
575.	3. 3 266 41.5	14 2 5 267	147.8	189.3





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			1						!	,		TONE FOUR DAM NO. O. T. C.S. MAKING MEST.		-1
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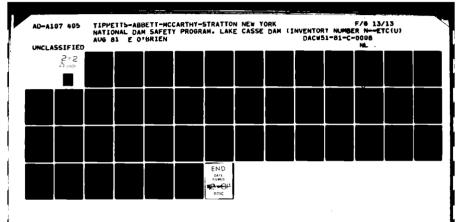
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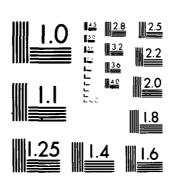
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#ULTI-PLAN ANALYSES TO 3E PEPFORMED  ###################################	. 0 0 0
#ULTI-PLAN ANALYSES TO 3E PEFFORMED  ***********************************	
SUB-AREA RUNDEF COMPUTATION  1 NORTH LAKE BASIN PUN-DFF  1 STAG  1 STAG  1 COMP  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO 0  1 TO	MULTI-PLAN ANALYSES TO NPLAN= 1 NRTIO= 4
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1 NORTH LAKE BASIN PUN-OFF  15140 1600 11APE JPLT JPRT INAME ISTAGE  1 HYDE SHE FROM TASPE RATIO ISNOW ISAME LOCA  1 HYDE SHE FREE SNAP TRSDA TRSDE RATIO ISNOW ISAME LOCA  2 PRECIP DATA  2 PRECIP DATA	designation and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis and an analysis an
ISTAG ICOMP IECON ITAPE JPLT JPRT INAME ISTAGE  IHYDG : UHG TAREA SNAP TPSDA TRSPC RATIO ISNOW ISANE LOCAN  SPFE PRS R12 R24 R4F R72 R96  COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE PROCRAM IS . & COMPUTED RY THE P	
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UNIT HYDROGRAPH DATA  TP= .98 CP= .63 NTA= 0	UNIT HYDROGRAPH DATA  1P= .98 CP= .63 NTA= 0
RECESSION DATA	STRTG= -1.00 GRCSNON DATA STRTG= -1.00 GRCSN1.00 RT10R* 1.50
UNIT PYPROGRAPH 15 END-OF-PERIOD ORDINATES, LAG"	1.00
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APPENDIX E

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